

# Use of Tuned Mass Damper for Vibration Control of Casing Construction under Seismic Excitation

**Bandila. Chandra Sekhar, Asst. Prof. P. Hanuma**

Department of Civil Engineering,  
Sri Sunflower college of Engineering & Technology,  
Challapali

**Abstract-** Latest things in development industry requests taller and lighter constructions, which are additionally more adaptable and having very low damping esteem. This builds disappointment prospects and furthermore issues according to functionality perspective. Presently a-days a few strategies are accessible to limit the vibration of the design, out of the few methods accessible for vibration control, idea of utilizing TMD is a more current one. This review was made to concentrate on the viability of utilizing TMD for controlling vibration of design. At initial a mathematical calculation was created to examine the reaction of a shear building fitted with a TMD. Then, at that point, one more mathematical calculation was created to examine the reaction of a 2D edge model fitted with a TMD. An aggregate of three stacking conditions were applied at the foundation of the construction. Initial one was a sinusoidal stacking, the subsequent one was relating to viable time history according to spectra of IS-1894 (Part - 1):2002 for 5% damping at rough soil with ( $PGA = 1g$ ) and the third one was 1940 El Centro Earthquake record with ( $PGA = 0.313g$ ). From the review it was tracked down that, TMD can be viably utilized for vibration control of designs. TMD was more viable while damping proportion of the construction is less. Step by step expanding the mass proportion of the TMD brings about slow decrement in the removal reaction of the design..

**Keywords -** TMD, ELCentro, PGA.

## I. INTRODUCTION

Vibration control is having its foundations principally in aviation related issues like following and pointing, and in adaptable space structures, the innovation immediately moved into structural designing and framework related issues, like the security of structures and scaffolds from outrageous heaps of tremors and winds.

The quantity of tall structures being constructed is expanding step by step. Today we can't have a count of number of low-ascent or medium ascent and tall structures existing on the planet. For the most part these constructions are having low regular damping.

So expanding damping limit of an underlying framework, or considering the requirement for other mechanical means to expand the damping limit of a structure, has become progressively normal in the new age of tall and very tall structures.

In any case, it ought to be made a normal plan practice to plan the damping limit into an underlying framework while planning the primary framework. The control of primary vibrations delivered by tremor or wind should be possible by different means like adjusting rigidities, masses, damping, or shape, and by giving inactive or dynamic counter powers. Until now, a few strategies for primary control have been utilized effectively and recently proposed techniques offer the chance of expanding applications and further developing proficiency. The determination of

a specific sort of vibration control gadget is administered by various elements which incorporate proficiency, minimization and weight, capital expense, working expense, upkeep necessities and security. Tuned mass dampers (TMD) have been generally utilized for vibration control in mechanical designing frameworks.

Lately, TMD hypothesis has been taken on to decrease vibrations of tall structures and other structural designing constructions. Dynamic safeguards and tuned mass dampers are the acknowledge of tuned safeguards and tuned dampers for underlying vibration control applications.

The inertial, versatile, and dissipative components in such gadgets are: mass, spring and dashpot (or material damping) for straight applications and their turning partners in rotational applications. Contingent upon the application, these gadgets are estimated from a couple of ounces (grams) to numerous tons. Different arrangements like pendulum safeguards/dampers, and sloshing fluid safeguards/dampers have additionally been acknowledged for vibration relief applications.

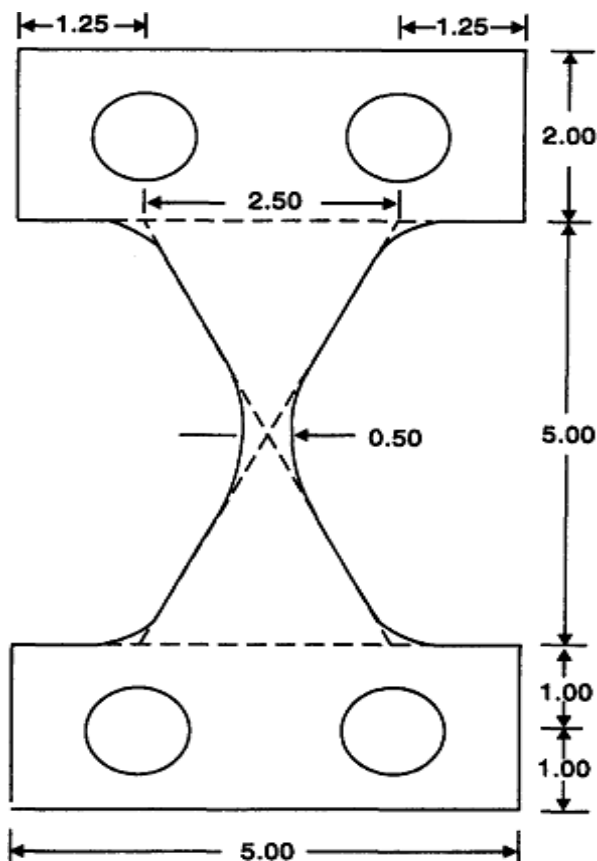


Fig 1. X-shaped ADAS device.

## II. CONCEPT OF TUNED MASS DAMPER SYSTEM

- Concept of tuned mass damper using two mass system
- Tuned Mass Damper theory for SDOF systems
- Undamped Structure: Undamped TMD

## III. RESULTS AND DISCUSIONS

### 1. Time Histories of Random Ground Acceleration:

A total of two random ground acceleration cases are considered for the analysis. The first is the compatible time history as per spectra of IS-1894 (Part -1):2002 for 5% damping at rocky soil. (PGA = 1.0g). The second is the 1940 El Centro Earthquake record (PGA = 0.313g).

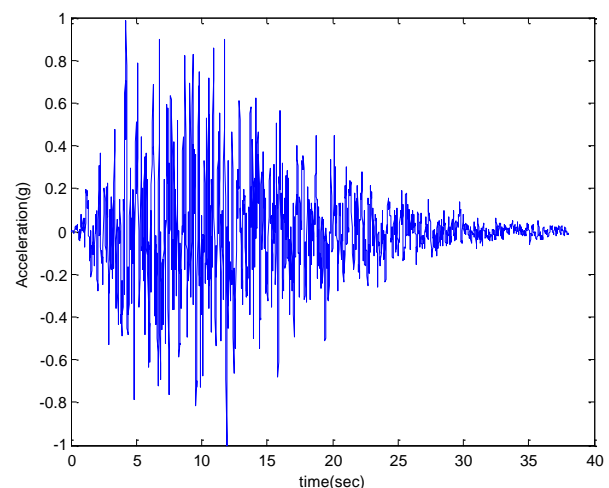


Fig 2. Compatible time history as per spectra of IS-1894 (Part -1):2002 for 5% damping at rocky soil.

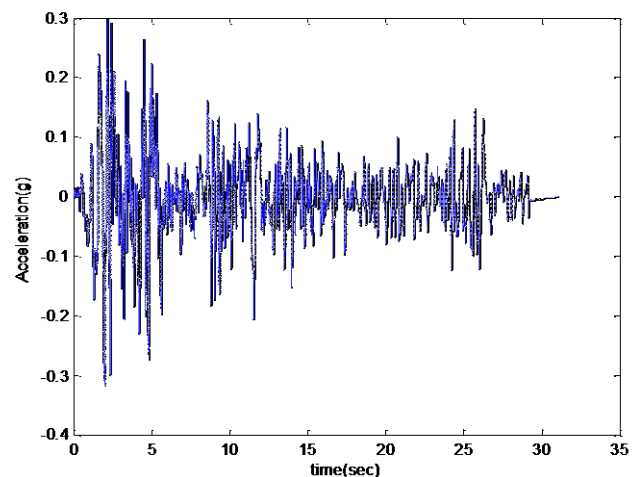


Fig 3. El Centro EQ History.

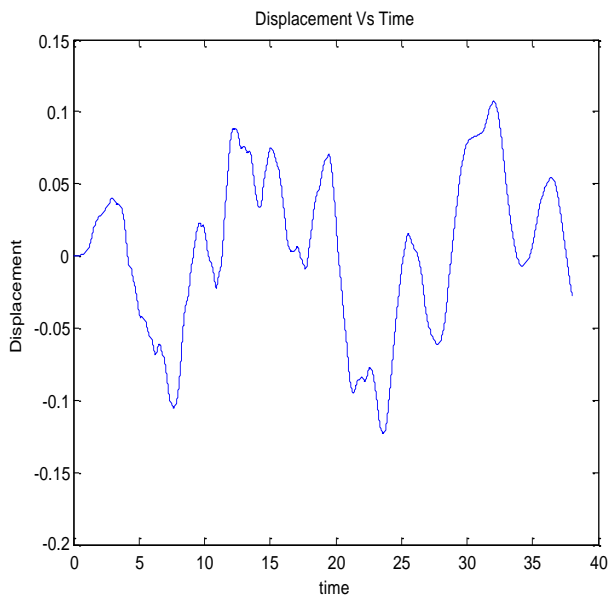


Fig 4. Response of shear building to Compatible time history as per spectra of IS-1894 (Part -1):2002 for 5% damping at rocky soil.

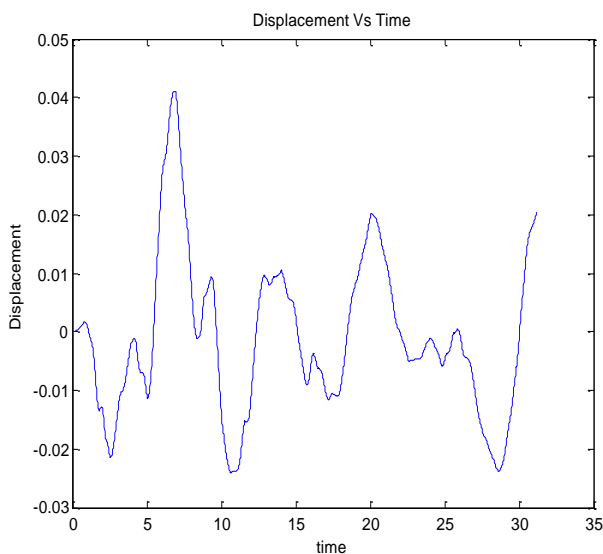


Fig 5. Response of shear building to the 1940 El Centro earthquake.

### III. CONCLUSIONS

Latest things in development industry requests taller and lighter designs, which are additionally more adaptable and having very low damping esteem. This builds disappointment prospects and furthermore, issues according to functionality perspective. A few procedures are accessible today to limit the vibration of the design, out of which idea of utilizing of TMD is one. This review is made to concentrate on the adequacy of utilizing TMD for controlling vibration of

design. A mathematical calculation was created to show the multi-story multi-level of opportunity building outline structure as shear working with a TMD. One more mathematical calculation is additionally evolved to break down 2D-MDOF outline structure fitted with a TMD.

An aggregate of three stacking conditions are applied at the foundation of the construction. Initial one is a sinusoidal stacking and the subsequent one relating to viable time history according to spectra of IS-1894(Part - 1):2002 for 5% damping at rough soil and the third one is 1940 El Centro Earthquake record (PGA = 0.313g).

Following conclusions can be made from this study:

- It has been found that the TMD can be effectively used to control vibration of the construction.
- TMD is more powerful in decreasing the relocation reactions of constructions with low damping proportions (2%). However, it is less compelling for structures with high damping proportions (5%).
- Applying the two quake loadings first is the one comparing to viable time history according to spectra of IS-1894 (Part-1): 2002 for 5% damping at rough soil and second being the 1940 El Centro Earthquake it has been found that expanding the mass proportion of the TMD diminishes the relocation reaction of the construction.

### IV. FURTHER SCOPE FOR STUDY

Both the structure and Damper model considered in this study are linear one; this provides a further scope to study this problem using a nonlinear model for TMD as well as for structure. The frame model considered here is two-dimensional, which can be further studied to include 3-dimensional structure model. Further scope, also includes studying the possibility of constructing Active TMD.

### REFERENCES

- [1] Baz (1998), — robust control of active constrained layer dampingII, Journal of Sound and Vibration 211 pp467–480.
- [2] Baz, (2000) — Spectral finite element modelling of wave propagation in rods using active constrained layer dampingII, Journal of Smart Materials and Structures9 pp372– 377.
- [3] Chattopadhyay, Q. Liu, H. Gu, (2000) — Vibration reduction in rotor blades using active composite

- box beam, American Institute of Aeronautics and Astronautics Journal 38 pp1125–1131.
- [4] Alexander Nicholas A, Schilder Frank(2009) —Exploring the performance of a nonlinear tuned mass damper, Journal of Sound and Vibration 319 pp 445–462
- [5] Alli H, Yakut O. (2005) —Fuzzy sliding-mode control of structures, Engineering Structures; 27(2).
- [6] Barkana I. (1987) — Parallel feed forward and simplified adaptive control International Journal of Adaptive Control and Signal Process; 1(2);pp95-109.
- [7] Barkana I. (2005) — Gain conditions and convergence of simple adaptive control Internet J Adapt Control Signal Process; 19(1);pp13-40.
- [8] Barkana I, Kaufman H. (1993) —Simple adaptive control of large flexible space structures". IEEE Trans Aerospace Electron System; 29(4).
- [9] Bitaraf Maryam, Ozbulut Osman E, Hurlebaus Stefan(2010) —Application of semi-active control strategies for seismic protection of buildings with MR dampers, Engineering Structures.